M Ferdows*1, Masahiro OTA1, Roushan Jahan1, MNA Bhuiyan2, Mubarak Hossain3

¹Department of Mechanical Engineering, Tokyo Metropolitan University, 1-1 Minami-osawa, Hachioji, Tokyo 192-0397, Japan ²Department of Mathematics, Dhaka University, Dhaka 1000, Bangladesh

ABSTRACT

To evaluate flood (Discharge or water level) as well as rainfall frequency of given returns period, it is essential that one probability distribution function be used as a standard. In many countries one distribution function is used as a standard but in Bangladesh various frequency distribution function are in use. The main objectives of this study are to compare the probability distribution function for the application on flood and rainfall frequency analysis in Bangladesh. To compare it, three widely used distributions have been used namely: (1) Log Normal (Two parameters, LN2 and three parameters, LN3); (2) Extreme value Type-1 (EV1) or Grumbel and (3) Log-person type-3 (LP3) distributions. For this purpose, 5 set data of annual maximum runoff of different main rivers in Bangladesh and 3-days and 5-days rainfall data of Bhola Island (8 sets) have been used. The parameters of the distributions have been estimated by using the method of moments and method of maximum likelihood.

Keywords: Flood frequency, Rainfall frequency, Probability distribution, Method of moments, Method of maximum likelihood

INTRODUCTION

Knowledge of the magnitude and probable frequency of recurrence of floods and rainfall is necessary for the proper design of hydraulic structures such as dams, bridges, culverts, levees, highways, sewage disposal plants, industrial building etc. Return periods are find as per design policy of such structures viz. flood of 50 years return period are evaluated for embankments and bridges, that of return period 100 years for barrage and culverts

In 1880-1890, Herchel and Freeman first applied the frequency analysis of stream flow data to flood studies by means of graphical procedure using flow duration curves. A large number of papers on the application of Fisher - Tippet theory of extreme values to flood frequency analysis were published [6]. Later, many other hydrologists worked on the extreme value theory to flood frequency analysis, [3, 7, 10, 11, 14, 15, 16]

The frequency analysis of discharge data of West Bengal rivers by means of graphical procedures using flow duration curves has been studied by [2, 13] and discussed various issues related to section of probability distribution function for flood frequency analysis. In the various rivers discharge data in Bangladesh, MPO (1986), [1, 4, 12] have been used different probability distribution and suggested that LP3 distributions are suitable for Bangladesh for the frequency analysis of discharge data. It was found from all of the above study that they have used only discharge data but in our study discharge as well as rainfall data have been used. The objective of this study is to compare the probability distribution function for the application of flood frequency as well as rainfall frequency analysis considering annul maximum runoff and rainfall data at different rivers and places in Bangladesh. In this study three widely used distributions have been compared by using 5 sets of annual maximum runoff data in different main rivers, 4 sets of 3days rainfall data and 4 sets of 5-days rainfall data of Bhola Island in Bangladesh. For the literature survey we have visited Surface water modeling center (SWMC), Bangladesh water development board (BWDB), Institute of flood control and drainage research (IFCDR), Roorkee university (India) etc. for several times and collected important literature related to the work mentioned above.

METHODOLOGY

Probability distribution functions used:

Probability distribution functions of discrete and continuos random variables are used to fit distributions in hydrology. There are many distributions that are found useful for the hydrological frequency analysis. The Bangladesh water development board which designs and implements all large-scale flood control projects uses the Gumbel distribution. A few departments and consulting firms use the log normal (LN) distribution. The log-Pearson type-3 (LP3) distribution has been used in the preparation of a national water plan. Three widely used probability functions were compared in this study. These three probability distribution function and the parameters involved in each function are given below.

Log-normal distribution (LN)

The probability density function of this distribution in the case of three parameters (LN3) is

$$f(x) = \frac{1}{(x \cdot \theta)\sigma_y \sqrt{2\pi}} \exp\left[\frac{-\left\{\ln\left(x \cdot \theta\right) - \mu_y\right\}^2}{2\sigma_y^2}\right], x > \theta$$
(1)

Where μ_y and σ_y are the mean and standard deviation of the natural logarithms of x and θ is a number.

The probability density function of this distribution in the case of two parameters (LN2):

$$f(x) = \frac{1}{x\sigma_y\sqrt{2\pi}} \exp\left[\frac{-(\ln x - \mu_y)^2}{2\sigma_y^2}\right]$$
(2)

Where σ_y and μ_y are parameters stated above.

Extreme Value Type-1 or Grrmbel Distribution (EV1)

The probability density function of this distribution is

$$f(x) = \exp\left[-\exp\left(-\frac{x-\xi}{\alpha}\right)\right]$$
(3)

Where f(x) s the non-excedence probability for the value of x, ξ is a location parameter and α is scale parameter.

Log Pearson Type-3 Distribution (LP3)

The probability density function of Person Type-3 distribution is

$$f(x) = \frac{1}{\alpha x \Gamma(\beta)} \left[\frac{x \cdot v}{\alpha} \right]^{\beta(1)} \exp\left[-\frac{x \cdot v}{\alpha} \right]$$
(4)

Where α , β and v are the shape, scale and location parameters to be estimated from the sample and $\Gamma(\beta)$ is the gamma function.

If the logarithm, lnx of a variable x are distributed as a Pearson Type-3 variable, then the variable x will be distributed as a Log Pearson Type-3 with probability density function

$$f(x) = \frac{1}{\alpha x \Gamma(\beta)} \left(\frac{\ln x \cdot v}{\alpha} \right)^{(\beta-1)} \exp\left[-\frac{x \cdot v}{\alpha} \right]$$
(5)

Where α , β and Γ are the parameters as before.

1.2 Method of estimating distribution function parameters

The estimation methods techniques are used for estimating various parameters from sample values in such a way that they depart from the population parameters to a minimum. For estimating the parameters from the sample of data, method of moments (MM) and the method of maximum likelihood (MML) have been used in this study. The maximum likelihood method is a standard statistical procedure used in fitting a variety of hydrological data [5, 8].

1.3 Data Used in this analysis

The annual maximum discharge and 3 days and 5-days rainfall data have been used in this study. The data have been found from the Bangladesh water development board and Institute of flood control and drainage research. For the computation of statistical probability distribution a total number of 5 sets discharge data and 8 sets of rainfall data have been selected for this study on various types of rivers and places in Bangladesh. The discharge data record covers up to the year 2000 and rainfall data covers up to 2000 in some cases as given in table-1. The length of annual maximum discharge data lies between 25 to 63 years and rainfall data lies between 11 to 35 years as given in table-2. Though there is some break in the period of observation, the data are assumed to be continuous in this study.

Table-1: Discharge data (5 sets) of 5 different rivers in Bangladesh.

Serial			
Number	Station & River	Period of Record	Number of Years
1.	273Bhairab Bazar (Meghna)	1964 to 2000	29
2.	46-9L Bahadurabad (Brahmaputra)	1956 to 1992	36
3.	91-9L Baruria (Ganges)	1966 to 1992	25
4.	90 Hardinage Bridge (Ganges)	1964 to 2000	63
5	229-5LMymensingh(Old Brahmaputra)	1965 to 1992	26

Table-2: Rainfall data (8 sets) of 4 different rainfall station in Bhola (3-days and 5-days)

Serial	Rainfall	Period of Record	Number of Years
Number	Station		
1.	Bhola city	1962 to 2000	35
2.	Burhanuddin	1962 to 1994	25
3.	Charfassion	1968 to 1978	11
4.	Daulatkhan	1961 to 1994	22

RESULTS AND DISCUSSION

For computations of the flood and rainfall frequency analysis by the distribution LN2, LN3, EV1 and LP3 have been developed and then used for this study. The application of above mentioned distributions have been used for the estimations of T year's events (viz. 2, 5, 10, 20, 50 and 100 years) through the method of moments and the method of maximum likelihood. All results are available in Appendices (in Appendix Al: Annual maximum discharge data of 5 sets in tabular from are given. In Appendix A2: Results of Discharge data are shown. B1: Rainfall data 3-days 4 sets and 5-days 4 sets in tabular form are given and Appendix B2: Results of Rainfall data are shown)

CONCLUSION

From this study, it is found that LP3 distribution give reasonably good results for the flood frequency as well as rainfall frequency analysis. It gives moderate or average results out of the other distributions.

It is conclude that one may use LP3 distribution for the flood as well as rainfall frequency analysis in Bangladesh. For the present study (only six months duration) the limited number of flood (maximum discharge) and rainfall data have been used. For more confirmation a large number of data can be used for the further study.

REFERENCE

- Chowdhury, J.U and Karim, M.A. (1993), Selection of Probability Function for Flood Frequency Analysis in Bangladesh. Institute of Flood Control and Drainage Research, BUET, Dhaka Final Report, RO1/93.
- [2] Cunnane, C. (1989) Statistical Distribution for Flood Frequency Analysis, World Meteorological Organization, Operational Hydrology Report No. 33, WMONo. 718, Secretarial of the World Meteorological Organization Geneva, Switzerland.
- [3] Chow V.T. David R Maidment, and Larry W. Mays (1992), Hand book of Applied Hydrology, McGraw-Hill book company, U.S.A., Chapter 12.
- [4] Flood Hydrology study (1992), AFP-25: Flood Medelling and Management, Report of Kruger Consult and ECOM. Flood Plan coordination Organization.dhaka.
- [5] Farago, T., and R.W.Katz., 1990. Extremes and design values in climatology. World Meteorological organization, TD-No.386, Geneva, Switzerland.

- [6] Gumbel, E.J (1941), Probability Interpretation of the Observed Return Periods of Floods, Trans. American Geophysics unions, vol. 22.
- [7] Johnson, N, L and Lotz, S. (1970), Distribution in Statistics, Continuous Univariate Distribution-1. A Wiley- and Son. New-Yorkk.
- [8] Kite,G.W,1977. Frequency and risk analysis in Hydrology. Water resource publications, Fort collins, co,224 p.
- [9] Master Plan Organization (1986), Floods and Storms, Technical Report, No. 11
- [10] Mutreja. K.N. (1986) Applied Hydrology, Chap-3 and Chap-13, Tata McGraw-Hill Publishing Company Limited, New Delhi.
- [11] Nash. J.E and Amoracho, J. (1986), The Accuracy of Predication of Flood of High return Period. Water Resources Research vol. 2 No. 2.
- [12] Rahman. M.R. and Ahmed. S.M.U. (1986), A Comparative Study of Flood Frequency Analysis in Bangladesh. Journal of the Institution of Engineer, Bangladesh. vol. 14 No.-2.Page 31-37.
- [13] Roy, S.K. (1985), Flood Frequency Studies for Bhagirathi River (West Bengal, Unesco Sponsored International Hydrology Course, University Roorkee-247667, India.
- [14] Subramanya, K. (1994), Engineering Hydrology, Chap-7, Tata McGraw-Hill Publishing Company Limited, New Delhi. (2nd edition)
- [15] Singh, V.P. (1992), Elementary Hydrology, Chapter-25, Hall of India Private Limited, New Delhi.
- [16] Sir William Hall Crow and Partmers Ltd. (1993) South West Area Water Resources Management project FAP4, Final report volume-3.

Appendix-A1 (I) (Detailed of discharge data, which have been used in this study, is given in the following tables) *Table 1:* Annual maximum discharges (cumec) with year. Period of record used from 1961 to 2000, Number of years 29, Station Bhairab Bazar (273). River: Meghna. Designation of data set is D1

Year	1961	1962	1963	1964	1965	1966	1967	1968	1969	1970
Maxi Discharge	13141	7590	9487	12300	12100	14400	12700	13300	11500	16400
Year	1972	1973	1974	1975	1976	1981	1982	1983	1984	1985
Maxi Discharge	11500	12400	19500	12700	16700	11200	13500	16000	13600	14300
Year	1986	1987	1988	1989	1990	1991	1992	1993	2000	
Maxi Discharge	11100	15200	19800	15500	11700	14500	12800	19900	12394	

Table 2: Annual maximum discharges (cumec) with year. Period of record used from 1956 to 1992, Number of years = 36, Station: Baruria (46-9L). River: Brahmaputra. Designation of data set is D2

Year	1956	1957	1958	1959	1960	1961	1962	1963	1964	1965
Maxi Discharge	60400	65500	71300	68500	64800	53800	59400	56400	63100	64200
Year	1966	1967	1968	1969	1970	1972	1973	1974	1975	1976
Maxi Discharge	68900	69600	62300	56000	75000	66600	67300	91100	52200	65600
Year	1977	1978	1979	1980	1981	1982	1983	1984	1985	1986
Maxi Discharge	66600	56600	66100	61200	66500	55900	56500	77000	63800	43100
Year	1987	1988	1989	1990	1991	1992				
Maxi Discharge	74000	98600	71100	64400	84100	67500				

Journal - The Institution of Engineers, Malaysia (Vol. 66, No. 1, March 2005)

Table 3: Annual	maximum discharges	(cumec) with yea	ar. Period of re	ecord used from	1966 to 1992	, Number of yea	urs= 25, S	tation
Baruria (91-9L).	River: Ganges. Desig	nation of data set	is D3					

Year	1966	1967	1968	1969	1970	1971	1973	1974	1975	1976
Maxi Discharge	81300	63600	80200	72700	84200	76600	90900	113000	93300	83500
Year	1977	1978	1980	1981	1982	1983	1984	1985	1986	1987
Maxi Discharge	81800	80400	109000	88200	89600	101000	107000	90500	81500	113000
Year	1988	1989	1990	1991	1992					
Maxi Discharge	132000	80000	83800	100000	726000					

Table 4: Annual maximum discharges 'Cumec' with year. Period of record: 1934-35 to 1995-96 except 1971-72, 96-2000.Number of years = 63,Station No = 90, Station: Harding Bridge, River = Ganges, Designation of data set is D4

Year	1934-35	1935-36	1936-37	1937-38	1938-39	1939-40	1940-41	1941-42	1942-43	
Maxi Discharge	46600	44000	45300	39400	47800	35900	39100	38300	44700	
Year	1943-44	1944-45	1945-46	1946-47	1947-48	1948-49	1949-50	1950-51	1951-52	
Maxi Discharge	43300	43300	42200	49100	51200	61100	52600	52600	42200	
Year	1952-53	1953-54	1954-55	1955-56	1956-57	1957-58	1958-59	1959-60	1960-61	
Maxi Discharge	52600	50900	58600	60300	60100	46200	56200	52700	48000	
Year	1961-62	1962-63	1963-64	1964-65	1965-66	1966-67	1967-68	1968-69	1969-70	
Maxi Discharge	73200	58700	56100	49000	36800	41900	50800	45200	55200	
Year	1970-71	1971-73	1973-75	1973-74	1974-76	1977-77	1976-78	1977-79	1978-80	
Maxi Discharge	48700	38200	50700	50700	51100	65400	51100	67900	36900	
Year	1980-81	1981-82	1982-83	1983-84	1984-85	1985-86	1986-87	1987-88	1988-89	
Maxi Discharge	57600	47900	61600	60000	56500	50600	53500	76000	72300	
Year	1989-90	1990-91	1991-92	1992-93	1993-94	1994-95	1995-96	1999	2000	
Maxi Discharge	31600	51300	56000	41900	44800	46100	49100	55019	60952	

Table 5: Annual maximum discharges (cumec) with year. Period of record used from 1964 to1992, Number of years 26, Station Baruria (228-5) River: Old Brahmaputra. Designation of data set is D5

Year	1964	1965	1966	1967	1968	1969	1970	1974	1975	1976
Maxi Discharge	2830	3230	3490	3000	2900	2770	3250	3820	3060	3210
Year	1977	1978	1979	1980	1981	1982	1983	1984	1985	1986
Maxi Discharge	3550	2770	2630	3340	2690	2470	2370	4780	3070	1930
Year	1987	1988	1989	1990	1991	1992				
Maxi Discharge	3230	4910	2180	2060	2900	1490				

Appendix-A2 (I) (Results and figures of discharge data)

Table 1: Comparison of flood frequency results using data D1 obtained by different methods of various desired return periods

Return periods (Years Distribution)	2	5	10	20	50	100
EV1 (cumec) MM: X _T MML: X _T	13584 13519	16101 15456	17767 16739	19366 17970	21435 19563	22985 20757
LN2 (cumce) MM and MML X_T	13742	15878	17125	18227	19553	20490
LN3 (cumce) MM: X _T MML: X _T	23584 8751	15741 10776	17120 11921	18411 12914	20053 14086	21271 14901
LP3 (cumce) MM (Direct): X_T MML (Indirect) X_T	13599 13487	16598 15641	17039 17111	18300 18556	19914 21985	21121 21985





Figure 1: Comparison of flood frequency using the results given in Table 1

Figure 2: Comparison of flood frequency using the results given in Table 2

Table 2: Co	mparison of floo	od frequency results	s using data D2 c	obtained by d	ifferent methods of	various desired	l return periods

Return periods (Years Distribution)	2	5	10	20	50	100
EV1 (cumec) MM: X_T MML: X_T	64555 64760	75023 75150	81954 82029	88603 88627	97208 97168	103657 103569
LN2 (cumce) MM and MML X_{T}	65357	74585	79918	84607	90214	94158
LN3 (cumce) MM: X_T MML: X_T	64715 60607	740064 69873	79939 75416	85388 80397	92247 86480	97293 90837
$ LP3 (cumce) MM (Direct): X_T MML (Indirect) X_T $	64762 64791	73929 74158	79694 80024	85059 85469	91852 92344	96882 97423

Return periods (Years Distribution)	2	5	10	20	50	100
EV1 (cumec) MM: X_T MML: X_T	87646 87477	103796 101229	114489 110334	124746 119068	138022 130373	147971 138844
LN2 (cumce) MM and MML X_T	88674	102446	110479	117583	126128	132166
LN3 (cumce) MM: X_T MML: X_T	87933 85637	101846 99731	110505 108856	118486 117481	128473 128543	135783 136813
LP3 (cumce) MM (Direct): X_T MML (Indirect) X_T	88014 87512	101568 101477	110012 110589	117823 119282	127655 130561	134897 139104

Table 3: Comparison of flood frequency results using data D3 obtained by different methods of various desired return periods





Figure 3: Comparison of flood frequency using the results given in Table 3

Figure 4: Comparison of flood frequency using the results given in Table 4

Table 4: Comparison of flood frequency results using data D4 obtained by different methods of various desired return periods

		1				1
Return periods (Years Distribution)	2	5	10	20	50	100
EV1 (cumec) MM: X _T MML: X _T	49350 49380	58513 58728	64581 64916	70401 70852	77934 78536	83579 84294
LN2 (cumce) MM and MML X_{T}	49984	58395	63342	67741	70358	76833
LN3 (cumce) MM: X _T MML: X _T	50081 45914	58462 54462	63324 59580	67611 64183	72751 69809	76373 73841
LP3 (cumce) MM (Direct): X _T			Does N	ot Exist		
MML (Indirect) X_T	87512	101477	110589	119282	130561	139104



Figure 5: Comparison of flood frequency using the results given in Table 5

Return periods (Years Distribution)	2	5	10	20	50	100
EV1 (cumec) MM: X_T MML: X_T	2883 2890	3668 3645	4188 4146	4686 4626	5332 5247	5815 5712
LN2 (cumce) MM and MML X_T	2906	3584	4000	4379	4849	5191
LN3 (cumce) MM: X _T MML: X _T	2918 2887	3594 3555	4000 3944	4364 4287	4810 4700	5130 4991
LP3 (cumce) MM (Direct): X _T			Does N	ot Exist		
MML (Indirect) X_T	2965	3619	3975	4272	4608	4831

Table 5: Comparison of flood frequency results using data D5 obtained by different methods of various desired return periods

Appendix-B1 (Rainfall data [(I) 3-days-4 sets (II) 5-days-4 sets])

Detailed of rainfall data, which have been used in this study, are given in the following tables.

Table 1: Annual maximum rainfall (3-days) in "mm" with year. Period of record used from 1962 to 2000.Number of years = 37, Name of station: Bhola. Designation of data set is 'R-1'

Year	1962	1963	1964	1965	1966	1967	1968	1969	1970
Maximum rainfall	226.6	258.8	293.2	159.5	281.7	212.1	226.8	380.2	192.8
Year	1971	1972	1973	1974	1975	1976	1977	1978	1979
Maximum rainfall	215.2	262.2	177.2	237.0	248.3	279.4	165.1	354.3	219.3
Year	1980	1981	1982	1983	1984	1985	1986	1987	1988
Maximum rainfall	212.9	235.0	205.5	236.2	251.4	256.1	464.8	292.1	267.7
Year	1990	1991	1992	1993	1994	1995	1996	1997	1998
Maximum rainfall	563.3	581.3	463.4	348.0	149.1	165	342.5	176.6	350.0
Year	2000								
Maximum rainfall	275								

Table 2: Annual maximum rainfall (3-days) in "mm" with year. Period of record used from 1962 to 1981 to 1994.Number of years = 25, Name of station: Borhanuddin, Designation of data set is 'R-2'

Year	1962	1963	1964	1965	1966	1967	1968	1969	1970
Maximum rainfall	219.7	33.4	295.1	215.1	351.5	310.0	302.8	505.5	271.7
Year	1971	1972	1973	1974	1975	1976	1977	1978	1979
Maximum rainfall	215.8	192.6	302.0	318.0	388.1	290.2	232.9	259.3	268.0
Year	1980	1981	1982	1983	1984	1985	1986	1987	1988
Maximum rainfall	165.1	176.5	295.1	180.4	202.5	638.0	132.0		

Table 3: Annual maximum rainfall (3-days) in "mm" with year. Period of record used from 1968 to 1978. Number of years = 11, Station: Charfession, Designation of data set is 'R-3'

Year	1968	1969	1970	1971	1972	1973	1974	1975	1976
Maximum rainfall	288.8	220.0	535.4	331.0	226.4	461.3	367.0	625.4	358.1
Year	1977	1978							
Maximum rainfall	252.4	294.6							

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Table 4: Annual maximum rainfall (3-days) in "mm"	with year. Period of record used from 1961 to 1994, Number of years =
22.Station: Designation of data set is 'R-4'	

Year	1961	1962	1965	1966	1967	1968	1969	1971	
Maximum rainfall	221.4	182.9	240.1	122.7	316.6	347.7	292.1	249.0	
Year	1972	1973	1974	1975	1976	1978	1979	1980	
Maximum rainfall	227.8	223.5	188.0	457.2	660.4	359.3	330.2	237.5	
Year	1981	1990	1991	1992	1993	1994			
Maximum rainfall	185.9	474.4	616.4	118.9	487.2	255.5			

Table 5: Annual maximum rainfall (5-days) in "mm" with year. Period of record used from 1962 to 2000. Number of years = 37, Station: Bhola. Designation of data set is 'R-5'

Year	1962	1963	1964	1966	1967	1968	1969	1970	
Maximum rainfall	250.7	287.8	319.4	228.3	282.2	305.1	294.9	407.7	
Year	1971	1972	1973	1974	1975	1976	1977	1978	
Maximum rainfall	221.8	264.2	311.2	202.6	286.9	291.7	329.2	181.6	
Year	1979	1980	1981	1982	1983	1984	1985	1986	
Maximum rainfall	478.8	258.9	329.8	245.4	240.0	370.8	251.4	269.2	
Year	1987	1988	1989	1990	1991	1992	1993	1994	
Maximum rainfall	272.2	438.2	375.8	182.8	267.7	32.0	598.6	275.3	
Year	1995	1996	1997	1998	1999	2000			
Maximum rainfall	330.1	350.5	274.4	525.0	361.5				

Table 6: Annual maximum rainfall (5-days) in "mm" with year. Period of record used from 1962 to 1982, 1990 to 1994.Number of years = 25.Station: Borhanuddin. Designation of data set is 'R-6'

Year	1962	1963	1965	1966	1967	1968	1969	1970	
Maximum rainfall	274.2	357.0	296.6	232.9	415.2	401.2	343.6	562.4	
Year	1971	1972	1973	1974	1975	1976	1977	1978	
Maximum rainfall	356.1	279.3	200.0	328.6	427.5	482.6	339.5	281.1	
Year	1979	1980	1981	1982	1990	1991	1992	1993	
Maximum rainfall	288.3	315.0	283.2	209.6	291.0	300.6	399.0	591.1	
Year	1994								
Maximum rainfall	175.1								

Table 7: Annual maximum rainfall (5-days) in "mm" with year. Period of record used from 1968 to 1978, Number of years = 11. Station: Charfession. Designation of data set is 'R-7'

Year	1968	1969	1970	1971	1972	1973	1974	1975	
Maximum rainfall	383.1	250.9	272.5	408.7	282.2	462.3	514.3	917.5	
Year	1976	1977	1978						
Maximum rainfall	41.9	406.1	351.5						

Table 8: Annual maximum rainfall (5-days) in "mm" with year. Period of record used from 1961 to 1982, 1990to 1994. Number of years = 22. Station: Daulatkhanh. Designation of data set is 'R-8'

Year	1961	1962	1965	1966	1968	1969	1970	1971	
Maximum rainfall	299.3	189.5	273.1	205.3	349.9	445.1	381.0	260.8	
Year	1972	1973	1974	1975	1977	1978	1979	1981	
Maximum rainfall	274.3	261.7	523.3	685.8	486.3	340.4	276.4	276.4	
Year	1982	1990	1991	1992	1993	1994			
Maximum rainfall	272.3	283.9	326.9	122.0	219.7	95.5			

Appendix-B2 (Results and figures of rainfall data)

Table 1: Comparison of Rainfall Frequency analysis results using data R1 obtained by different methods of various desired return

Return periods Distributions	2	5	10	20	50	100
LN2 (mm) MM and MML: X_T	259.7	353.7	415.7	475.0	552.0	610.0
LN3 (mm) MM:X _T MML: X _T	257.3 251.3	350.3 342.5	414.1 412.3	476.5 485.5	559.4 589.7	623.3 674.8
$Lp3 (mm) MM (direct) : X_T MM (indirect) : X_T MML : X_T$	258.1 248.9 251.0	349.0 338.0 338.0	411.2 409.5 407.0	472.2 488.5 480.5	553.8 608.2 588.9	617.1 712.7 681.3
$EV1 (mm) MM : X_T MML : X_T$	261.6 259.2	368.4 336.8	439.1 388.2	507.0 437.2	594.8 501.2	660.6 549.0







Figure 2: Comparison of rainfall frequency analysis using the results given in Table 2

Table 2: Comparison	on of Rainfall F	requency analysis	results using d	ata R2 obtain	ed by different	methods of vario	us desired return
perious (ill years)							

Return periods Distributions	2	5	10	20	50	100
LN2 (mm) MM and MML: X_T	263.5	360.8	425.2	487.0	567.4	628.2
LN3 (mm) MM:X _T MML: X _T	259.9 260.7	355.7 355.4	422.6 421.6	489.0 487.6	578.3 576.3	647.9 645.6
$\label{eq:main_state} \begin{array}{c} Lp3 \ (mm) \\ MM \ (direct): X_{\scriptscriptstyle T} \\ MM \ (indirect): X_{\scriptscriptstyle T} \\ MML: X_{\scriptscriptstyle T} \end{array}$	261.3 257.4	354.1 351.7	418.7 422.0	483.1 495.6	570.4 600.7	639.1 687.8
$EV1 (mm) MM : X_T MML : X_T$	266.0 264.7	379.5 350.2	454.7 406.9	526.8 461.2	620.1 531.6	690.0 584.3

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Return periods Distributions	2	5	10	20	50	100
LN2 (mm) MM and MML: X_{T}	326.3	449.8	532.0	611.1	714.3	792.5
LN3 (mm) MM:X _T MML: X _T	334.7 303.6	458.7 449.9	534.3 579.9	603.0 730.7	687.9 966.8	749.4 1176.6
$Lp3 (mm) MM (direct) : X_T MM (indirect) : X_T MML : X_T$	316.2 296.3	444.7 440.1	543.4 579.3	649.0 756.3	803.4 1069.4	933.8 1387.0
EV1 (mm) MM : X_T MML : X_T	331.8 326.5	494.5 438.2	602.2 512.2	705.5 583.2	839.3 675.1	939.5 743.9

Table 3: Comparison of Rainfall Frequency analysis results using data R3 obtained by different methods of various desired return periods





Figure 3: Comparison of rainfall frequency analysis using the results given in Table 3

Figure 4: Comparison of rainfall frequency analysis using the results given in Table 4

Table 4: Comparison of	f Rainfall Frequency	analysis results	using data R	4 obtained by	different	methods of various	s desired return
periods (in years)		·	•				

Return periods Distributions	2	5	10	20	50	100
LN2 (mm) MM and MML: X_{T}	274.8	500.6	685.0	887.4	1187.7	1442.3
LN3 (mm) MM:X _T MML: X _T	275.8 274.1	504.0 468.9	688.4 641.1	889.5 840.1	1185.8 1150.5	1435.7 1425.4
$\label{eq:linear} \begin{array}{c} Lp3 \ (mm) \\ MM \ (direct) : X_T \\ MM \ (indirect) : X_T \\ MML : X_T \end{array}$	258.9 273.1	438.8 452.4	628.2 615.8	886.2 814.1	1380.2 1146.1	1919.8 1464.2
$EV1 (mm)$ $MM : X_{T}$ $MML : X_{T}$	311.3 307.3	615.0 464.6	816.1 568.8	1009.0 668.7	1258.7 798.0	1445.8 894.9

Return periods Distributions	2	5	10	20	50	100
LN2 (mm) MM and MML: X_{T}	276.9	365.0	421.8	475.3	543.7	594.6
LN3 (mm) MM:X _T MML: X _T	282.1 225.1	369.9 310.2	422.4 357.4	469.6 398.0	527.2 445.2	568.4 477.8
Lp3 (mm) MM (indirect) : X _T	328.6	346.3	347.1	353.9	377.5	409.0
$\begin{array}{l} EV1 \ (mm) \\ MM : X_{T} \\ MML : X_{T} \end{array}$	277.1 281.7	377.0 393.2	443.2 467.1	506.6 537.9	588.7 629.5	650.3 698.2

Table 5: Comparison of Rainfall Frequency analysis results using data R5 obtained by different methods of various desired return periods (in years)





Figure 5: Comparison of rainfall frequency analysis using the results given in Table 5

Figure 6: Comparison of rainfall frequency analysis using the results given in Table 6

Return periods Distributions	2	5	10	20	50	100
LN2 (mm) MM and MML: X_{T}	322.6	414.4	472.5	526.4	594.6	644.9
LN3 (mm) MM: X_T MML: X_T	323.9 321.9	415.7 414.7	472.7 474.4	525.0 530.6	590.4 602.2	638.1 655.6
Lp3 (mm) MM (indirect) : X _T	321.7	414.2	474.1	530.7	603.4	658.0
EV1 (mm) MM : X_T MML : X_T	321.7 320.6	428.4 412.0	498.9 472.5	566.7 530.6	654.3 605.7	720.0 662.0

Table 6: Comparison of Rainfall Frequency analysis results using data R6 obtained by different methods of various desired return periods (in years)

Return periods Distributions	2	5	10	20	50	100
LN2 (mm) MM and MML: X_{T}	389.0	550.7	660.5	676.4	908.6	1016.9
LN3 (mm) MM:X _T	381.8	538.7	653.0	769.5	930.3	1058.6
Lp3 (mm) MM (direct) : X_T MM (indirect) : X_T	386.0 357.5	533.7 493.1	640.3 628.4	749.4 802.7	901.5 1115.2	1024.3 1436.2
$EV1 (mm) MM : X_T MML : X_T$	398.5 393.4	612.6 513.5	754.3 593.0	890.2 669.3	1066.2 680.5	1198.1 842.0

Table 7: Comparison of Rainfall Frequency analysis results using data R7 obtained by different methods of various desired return periods (in years)



Figure 7: Comparison of rainfall frequency analysis using the results given in Table 7



Figure 8: Comparison of rainfall frequency analysis using the results given in Table 8

Table 8: Comparison of Rainfall Frequency	analysis results using	data R8 obtained by	different methods of	various desired return
periods (in years)				

Return periods Distributions	2	5	10	20	50	100
LN2 (mm) MM and MML: X_{T}	286.0	403.8	483.6	561.2	663.5	741.9
LN3 (mm) MM:X _T MML: X _T	290.9 293.4	409.9 411.1	486.0 485.0	557.4 553.4	648.3 639.5	715.8 702.7
Lp3 (mm) MM (indirect) : X _T	301.3	416.0	478.0	529.7	585.7	621.6
EV1 (mm) MM : X_T MML : X_T	291.2 290.7	431.4 409.5	524.2 488.2	613.2 563.6	728.5 661.3	814.8 734.5